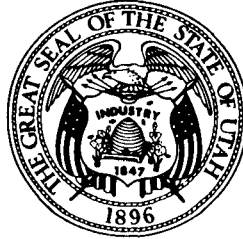


STATE OF UTAH
DEPARTMENT OF NATURAL RESOURCES

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SEEPAGE STUDY OF THE SEVIER VALLEY-PIUTE CANAL,
SEVIER COUNTY, UTAH

by

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Prepared by
the United States Geological Survey
in cooperation with
the Utah Department of Natural Resources
Division of Water Rights

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ENGLISH-TO-METRIC CONVERSION FACTORS

Most numbers are given in this report in English units followed by metric units. The conversion factors are shown to four significant figures. In the text, however, the metric equivalents are shown only to the number of significant figures consistent with the accuracy of the number in English units.

| <u>English</u> <u>Units</u> (Multiply) | <u>Abbreviation</u> | (by) | <u>Metric</u> <u>Units</u> (to obtain) | <u>Abbreviation</u> |
|--|-------------------------|---------|--|------------------------|
| Cubic feet per second | ft ³ /s | 0.02832 | Cubic meters per second | m ³ /s |
| Cubic feet per second per mile | (ft ³ /s)/mi | .01760 | Cubic meters per second per kilometer | (m ³ /s)/km |
| Feet | ft | .3048 | Meters | m |
| Miles | mi | 1.609 | Kilometers | km |

Water temperature is given in degrees Celsius (°C), which can be converted to degrees Fahrenheit (°F) by the following equation:

$$^{\circ}\text{F} = 1.8(^{\circ}\text{C}) + 32$$

SEEPAGE STUDY OF THE SEVIER VALLEY-PIUTE
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ABSTRACT

A study of the gains or losses of the Sevier Valley-Piute Canal from near Joseph to near Aurora, Sevier County, Utah, was made to aid in water allocation for the canal system. Four sets of seepage measurements were made in 1976, with the three most representative being used in the analysis. Adjustments for fluctuations in flow in the canals were made from information obtained from water-stage recorders operated at selected locations along the canal during the time of each seepage run.

The study showed a net loss of 13 cubic feet per second (0.37 cubic meter per second). During the seepage runs, an average of 198 cubic feet per second (5.6 cubic meters per second) entered the canal, thus the net loss was 6.6 percent of the available water.

INTRODUCTION

This report gives the results of the third of a series of canal-seepage studies in Utah. The study is part of the Statewide water-resources program conducted by the U.S. Geological Survey in cooperation with the Utah Department of Natural Resources, Division of Water Rights. Information on individual canal gains or losses by seepage is needed particularly by the Division of Water Rights when allocating water along canal systems. It is desirable also to know the sections of the canal where water is lost or gained, and this information is best obtained by detailed gaging of canals.

This report describes the study of the Sevier Valley-Piute Canal from near Joseph to near Aurora, Sevier County, Utah. The total length of canal studied is 29.6 mi (47.6 km). The Sevier Valley Canal (fig. 1) diverts from the left bank of the Sevier River about 2 mi (3.2 km) south of Joseph and has a capacity of about 350 ft³/s (9.9 m³/s) at the head. At site M21, about 4 mi (6.4 km) north of Richfield, the name of the canal changes to Piute Canal.

METHODS OF INVESTIGATION

A reconnaissance was made in the fall of 1975. The entire length of the canal was examined for: (1) the locations of main canal controls, drops, turnouts, or other diversion structures and bridges crossing the canal; (2) the general condition of the canal (for example, whether the canal had been recently cleaned or other maintenance had been performed); and (3) the locations of areas of natural and irrigation return flow to the canal.

Using the information from the reconnaissance and the geology of the area as described by Young and Carpenter (1965), the canal was divided into reaches and measuring sites were located within each reach. Water-stage recorders were installed at both ends of the canal and at the dividing point between each reach. A total of seven water-stage recorders were installed. Because of the size of the canal it was necessary to locate measuring sites at existing bridges or to construct measuring bridges.

Four sets of seepage measurements were made on the canal during 1976, covering the periods May 12 and 13, June 22 and 23, July 27 and 28, and September 14 and 15. The number of measuring sites on the canal was restricted because the depth of water prohibited wading measurements.

Prior to starting each set of seepage measurements, all personnel made calibration measurements with the current meters they were to use. Each person was assigned a reach for each day in which he could complete the required number of measurements. In each reach, measurements were made at all selected measuring sites, including both ends of the reach, all turnouts, and all inflow points. Figure 1 shows the sites where a measurement (or estimate) was made during at least one set of seepage measurements. The date, time of measurement, discharge at each measuring site, and the temperature and specific conductance of the water are shown in table 2.

The numbers used for the turnouts in figure 1 (for example, T10 or T10A) were taken from numbers marked on the headgate of the turnout. For a site without a marked number on the headgate, a designation such as T(I) was used. There is a duplication of turnout numbers because the turnouts were numbered beginning with site M1 in a downstream direction to site M21. Then, because the canal name changes at site M21, the turnouts were again numbered beginning with 1 in a downstream direction.

Continuous water-stage records were obtained at the head and tail of each reach, with the exception of site M30 where they were only obtained during the May measurements; the gage site was destroyed after the May measurements were made. The records of gage heights at recorders for the period of the May, June, and July measurements are shown in figure 2.

SEEPAGE MEASUREMENTS

The results of the seepage measurements, expressed in gain or loss along the canals, are given in table 1. The procedures used to obtain these results are described in the following pages.

A computation was made of the flow that would be expected at each main canal-measuring site, assuming no losses or gains. Beginning with the flow at the head of each reach and proceeding in a downstream sequence, all turnout flows were subtracted and all measured inflows were added. The computed value at each site was then adjusted for fluctuations in canal flow which originated above the reach being analyzed. Information required to make this adjustment is the change in flow with time at the head of the reach, the time of measurements at the head of the reach and the downstream measuring site, and the time required for passage of water from the head of the reach to the downstream site.

The change in flow with time at the head of the reach was determined from the recorded gage heights and the discharge measurements at the head of each reach. The times of the two measurements are available from table 2, and the time of travel between the two points was determined from the stage recorders at the ends of each reach.

As an example, assume that the measurement at the head of the reach was $200 \text{ ft}^3/\text{s}$ ($5.66 \text{ m}^3/\text{s}$) at 0800 hours, the measurement at the downstream measuring site was made at 1000 hours, the time required for flow to travel between the two sites is 1 hour, and the discharge at the head of the canal was dropping at the rate of $5 \text{ ft}^3/\text{s}$ ($0.142 \text{ m}^3/\text{s}$) per hour. To make the adjustment, the travel time is subtracted from the time of the downstream measurement (1000 hours - 1 hour = 0900 hours) to give a comparable time for flow at the head of the canal. From the gage-height records and the measurements available for the head of the canal, the flow at 0900 hours was calculated as $195 \text{ ft}^3/\text{s}$ ($5.52 \text{ m}^3/\text{s}$), or an adjustment of $-5 \text{ ft}^3/\text{s}$ ($0.142 \text{ m}^3/\text{s}$). This adjustment was then applied to the computed value of the downstream measuring site.

The computed value was then subtracted from the measured value to determine the amount of gain or loss between the head of the canal and the downstream measuring site. The amount of gain or loss was then plotted as a function of distance downstream from the canal head. This was done for each main canal-measuring site for each set of measurements.

In some instances, depending on the rate of gain or loss shown on these plots, or if the plotted points showed large amounts of scatter, the canal was segmented into shorter reaches. The data for each of the newly defined reaches were then plotted in figure 3, with the gain or loss at each main canal-measuring site plotted as a function of distance from the head of the reach. A straight line was fitted through the plotted points for each reach, and the amount and rate of gain or loss from the reach were determined from this line. The amount and rate of gain or loss by reach are shown in table 1.

Within a given reach, the amount of gain or loss varied in each set of seepage measurements and among the several sets of measurements. This variation is shown by the scatter of the plotted points in figure 3. The scatter is attributed to one or more of the following: poor measuring conditions, changes in the rate of seepage loss from the canal, changes in the rate of seepage return to the canal of ground and irrigation water, the inability to adjust completely for fluctuations in the amount of flow within a given reach, and the possibility that a water user changed the flow in his turnouts during the time of the measurements.

The results presented are based on only the seepage measurements made during May, June, and July. Because of a shortage of water, the flow in the canal was reduced earlier than anticipated, and it was only about 40 ft³/s (1.1 m³/s) on September 14 and about 20 ft³/s (0.6 m³/s) on September 15. Plots of the data from the September measurements indicated undetectable gains or losses. A check of the previous 5 years of flow records for this canal indicated that on 80 percent of the days of flow, the flow was greater than 50 ft³/s (1.4 m³/s). Therefore, it was decided to omit the September measurements from the final analysis.

EVALUATION OF THE CANAL SYSTEM

Most reaches that were studied had small to moderate gains or losses. The study showed a net loss of 13 ft³/s (0.37 m³/s) (table 1). During the seepage runs an average of 198 ft³/s (5.6 m³/s) entered the canal, thus the net loss was 6.6 percent of the available water. Following is a brief description of each reach studied and its calculated gain or loss. The geological descriptions are from Young and Carpenter (1965).

Reach M1-M2.--Site M1 is about 0.5 mi (0.8 km) below where the canal diverts from the Sevier River. Reach M1-M2 is constructed in alluvium composed of poorly to well sorted clay, silt, sand, gravel, and boulders; the upper half of the reach is only a few feet above the adjacent flood plain. This reach had a calculated gain of 1 ft³/s (0.03 m³/s).

Reach M2-M5.--Reach M2-M5 is constructed in alluvium composed of poorly to well sorted clay, silt, sand, gravel, and boulders. It runs through the town of Joseph and through several farms. This reach had a calculated loss of 4 ft³/s (0.11 m³/s).

Reach M5-M6.--Reach M5-M6 is constructed for the most part in alluvium composed of poorly to well sorted clay, silt, sand, gravel, and boulders. The lower part of the reach is constructed in fanglomerate deposits consisting of silt, sand, gravel, cobbles, and boulders derived from adjacent highlands. This reach had a calculated gain of 1 ft³/s (0.03 m³/s).

Reach M6-M9.--Reach M6-M9 for a short distance below site M6 is constructed in fanglomerate deposits. The rest of the reach, which runs along the Elsinore fault, is constructed primarily along the contact of

volcanic rocks (above the canal) and alluvium (below the canal). The lower end of the reach runs above the town of Elsinore, and it is lined with a considerable number of trees. This reach had a calculated loss of $5 \text{ ft}^3/\text{s}$ ($0.14 \text{ m}^3/\text{s}$).

Reach M9-M10.--Reach M9-M10 is constructed in alluvium. The area above the canal includes several irrigated fields. This reach had a calculated gain of $5 \text{ ft}^3/\text{s}$ ($0.14 \text{ m}^3/\text{s}$).

Reach M10-M11.--Reach M10-M11 is constructed in alluvium; some irrigated fields lie above the canal at the upper end of the reach, and considerable vegetation grows along the lower end of the reach. Site M11 turned out to be a poor measuring site; thus, the results for this reach (little or no net gain or loss) are considered to be poor.

Reach M11-M16.--Reach M11-M16 is constructed along the Elsinore fault, primarily along the contact of volcanic rocks (above the canal) and alluvium (below the canal). The lower end of this reach runs adjacent to the city of Richfield. There was no calculated net gain or loss for this reach.

Reach M16-M21.--Reach M16-M21 is constructed, for the most part, in alluvium; but it crosses the Elsinore fault in two places, and for about 0.75 mi (1.2 km) near site M19 it is underlain by the Flagstaff Limestone. This reach had a calculated loss of $10 \text{ ft}^3/\text{s}$ ($0.28 \text{ m}^3/\text{s}$), or $2.3 (\text{ft}^3/\text{s})/\text{mi}$ [$0.04 (\text{m}^3/\text{s})/\text{km}$], which was the highest loss rate of any reach studied.

Reach M21-M26.--Reach M21-M26 is constructed in alluvium. Site M21 marks the beginning of the Piute Canal. There was no calculated net gain or loss for this reach.

Reach M26-M28.--Reach M26-M28 is constructed in alluvium. This reach had a calculated loss of $1 \text{ ft}^3/\text{s}$ ($0.03 \text{ m}^3/\text{s}$).

Reach M28-M30.--Reach M28-M30 is constructed in alluvium. There was no calculated net gain or loss for this reach.

SUMMARY

A study of 29.6 mi (47.6 km) of the Sevier Valley-Piute Canal showed that seepage gains or losses were small to moderate. The average flow at the head of the canal during the seepage runs was $198 \text{ ft}^3/\text{s}$ ($5.6 \text{ m}^3/\text{s}$). The study showed a net loss of $13 \text{ ft}^3/\text{s}$ ($0.37 \text{ m}^3/\text{s}$), which was 6.6 percent of the available water.

REFERENCE CITED

Young, R. A., and Carpenter, C. H., 1965, Ground-water conditions and storage in the central Sevier Valley, Utah: U.S. Geol. Survey Water-Supply Paper 1787.

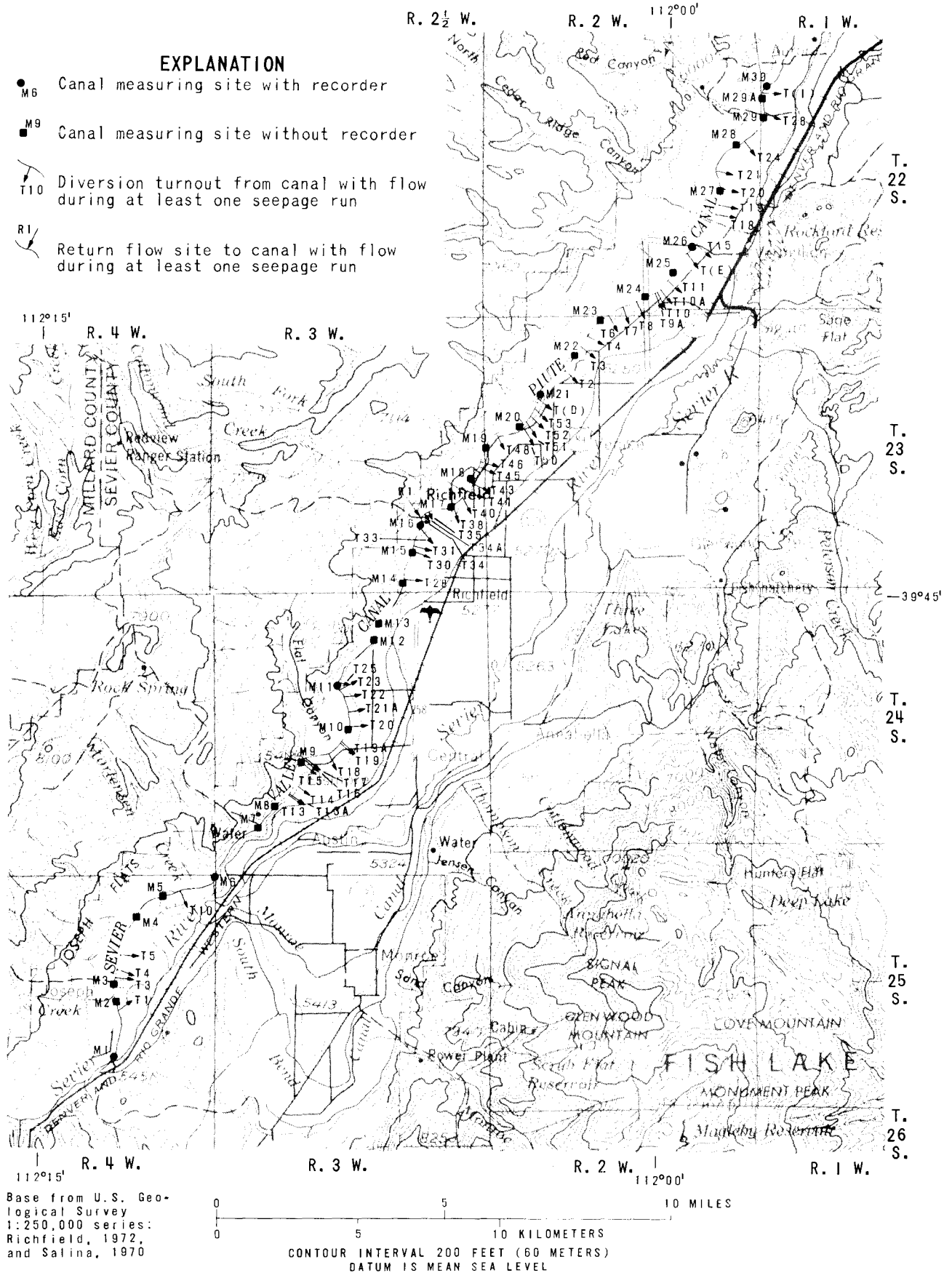


Figure 1.—Map of the Sevier Valley-Piute Canal showing measuring sites.

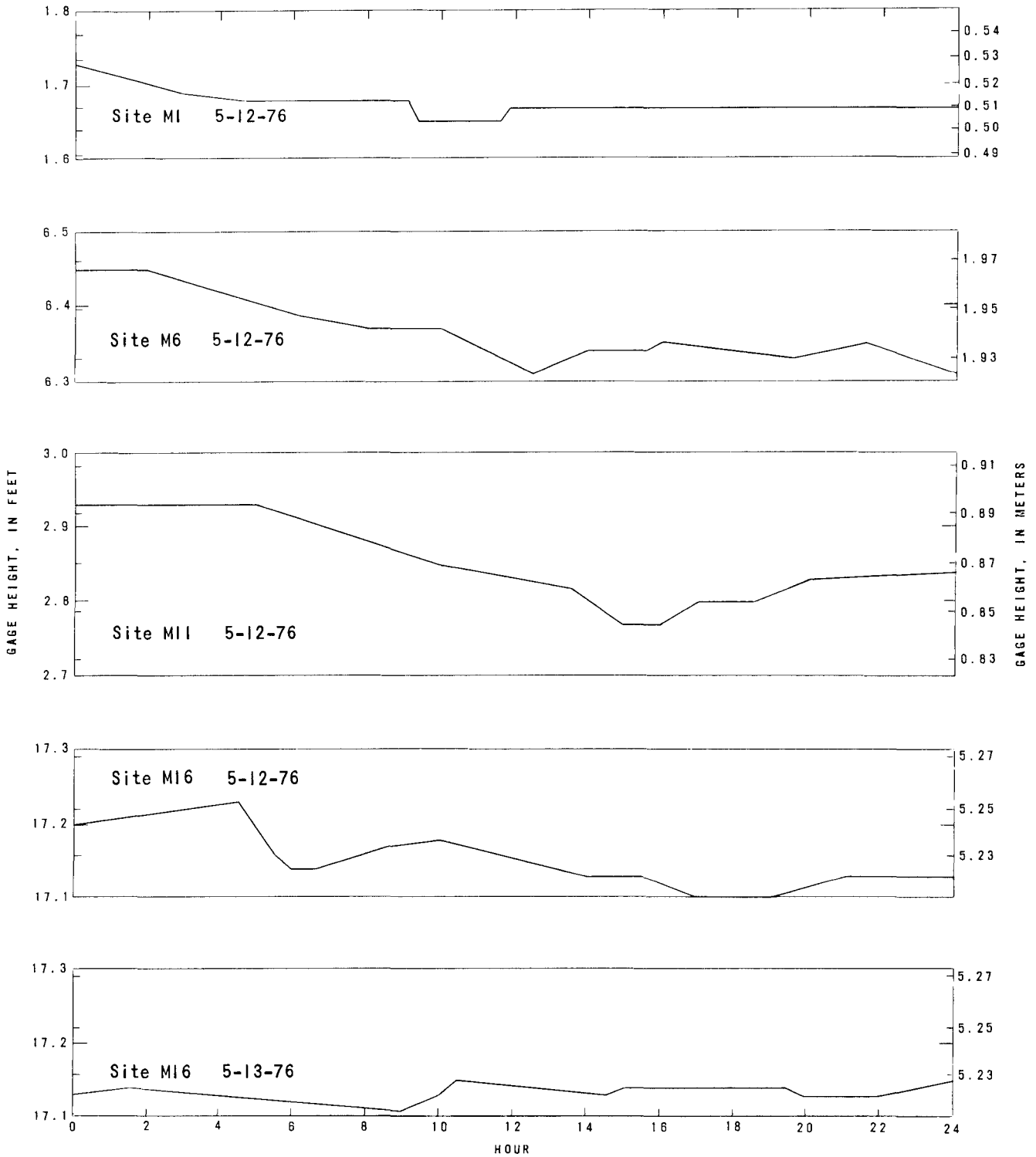


Figure 2.— Gage heights at recorders during seepage runs.

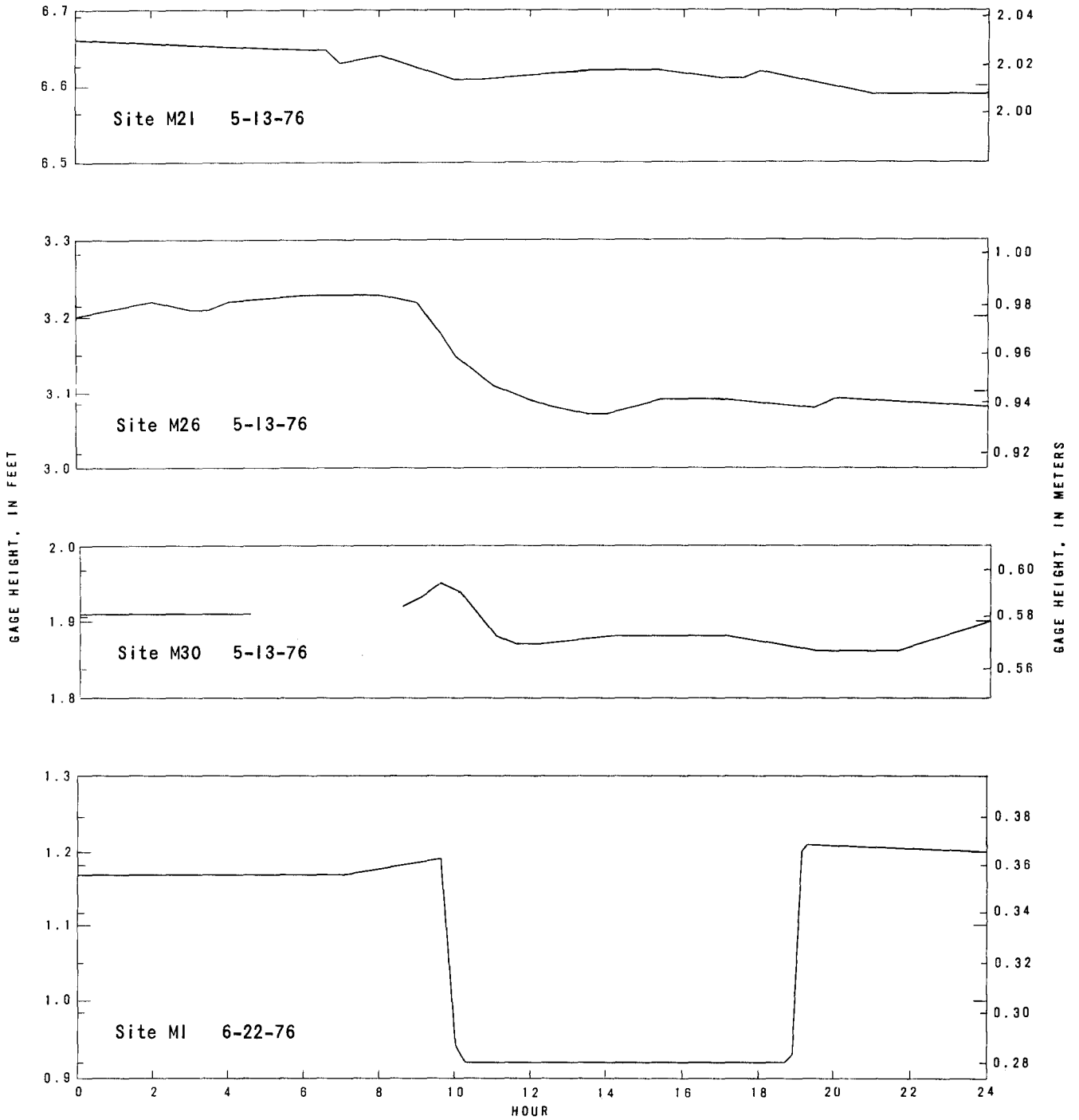


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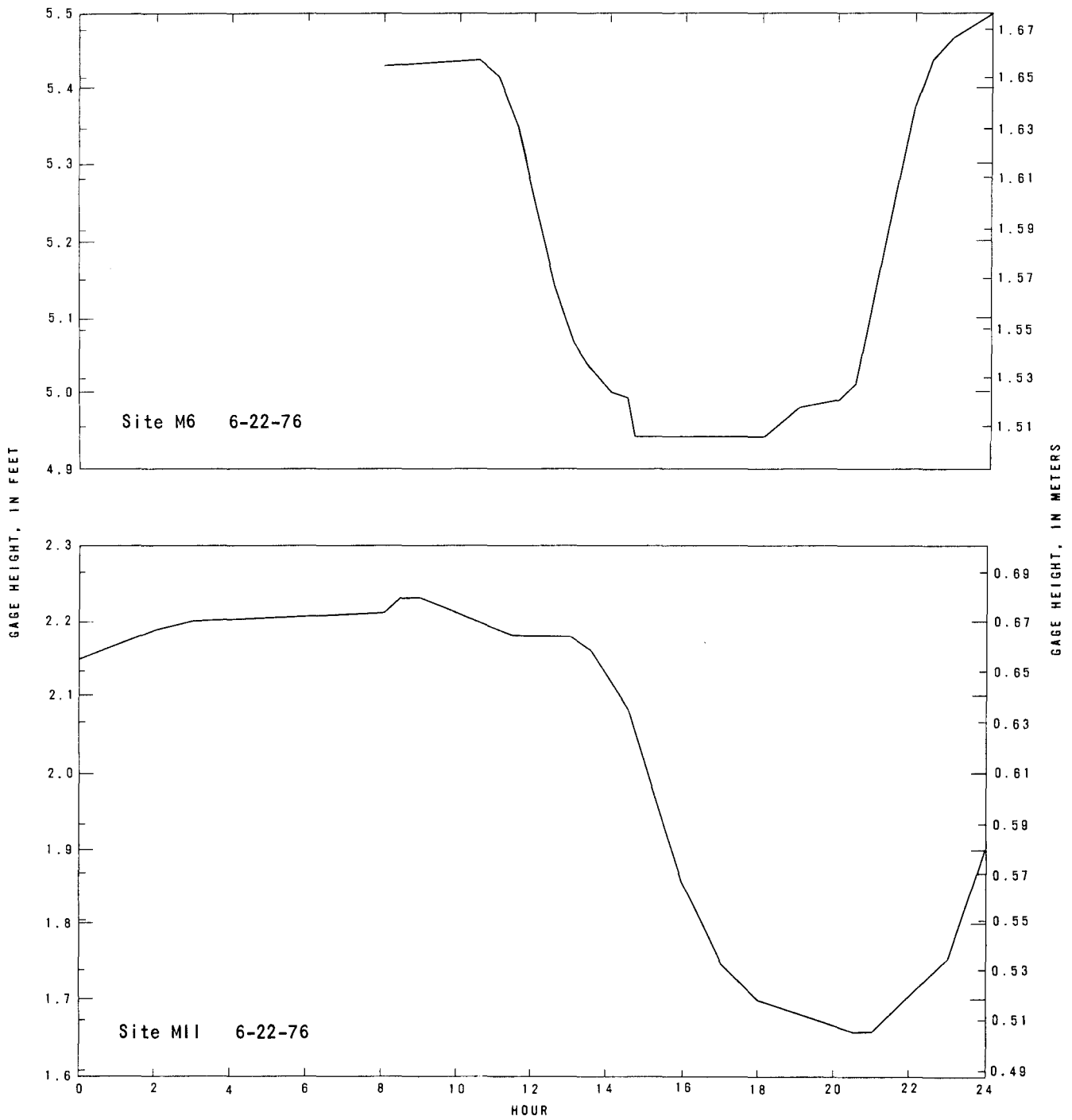


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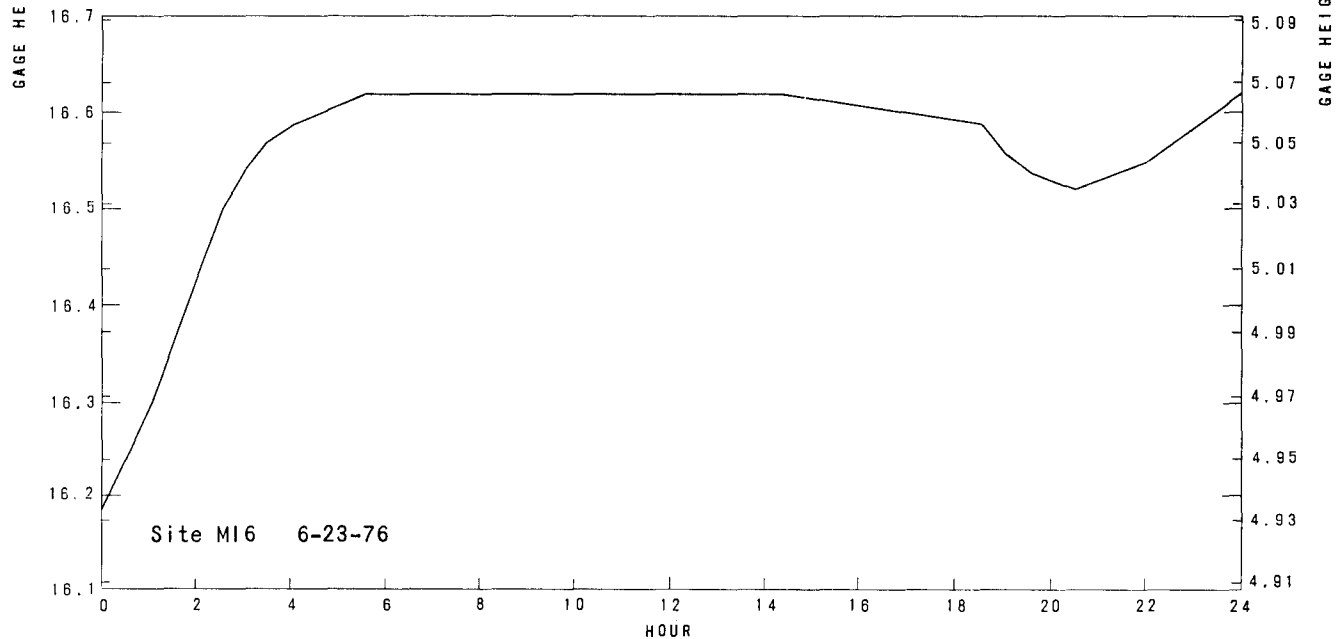
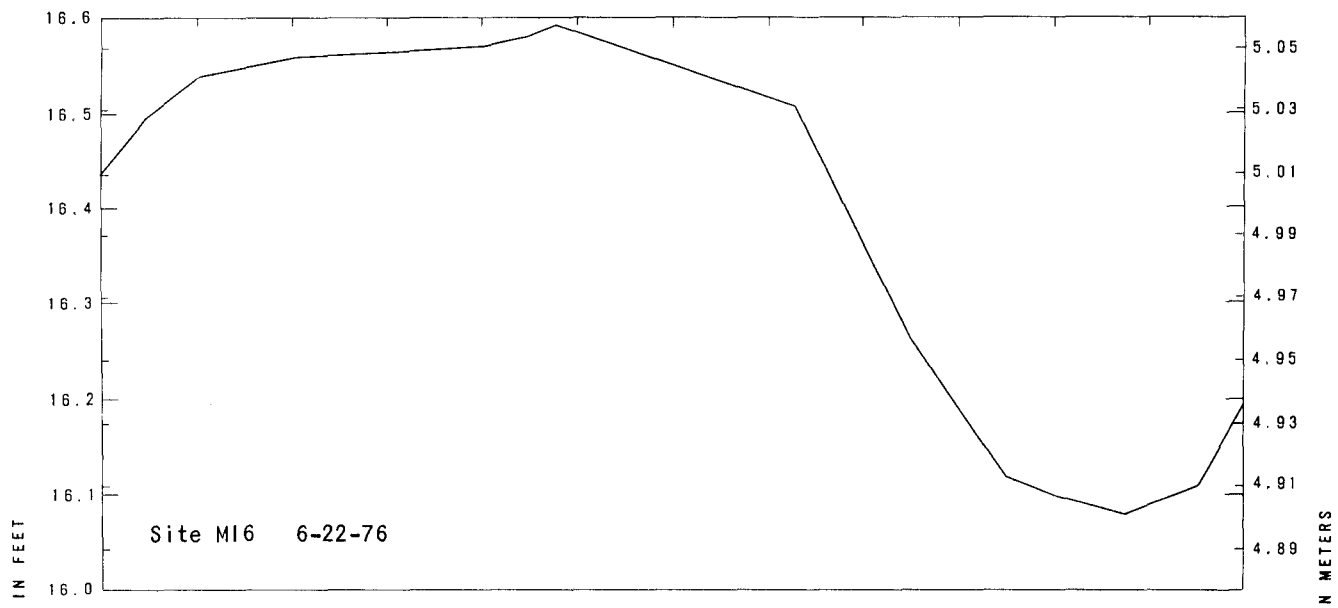


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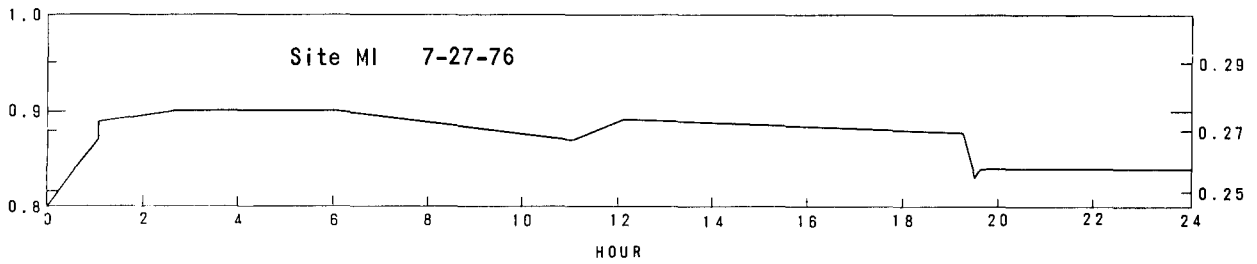
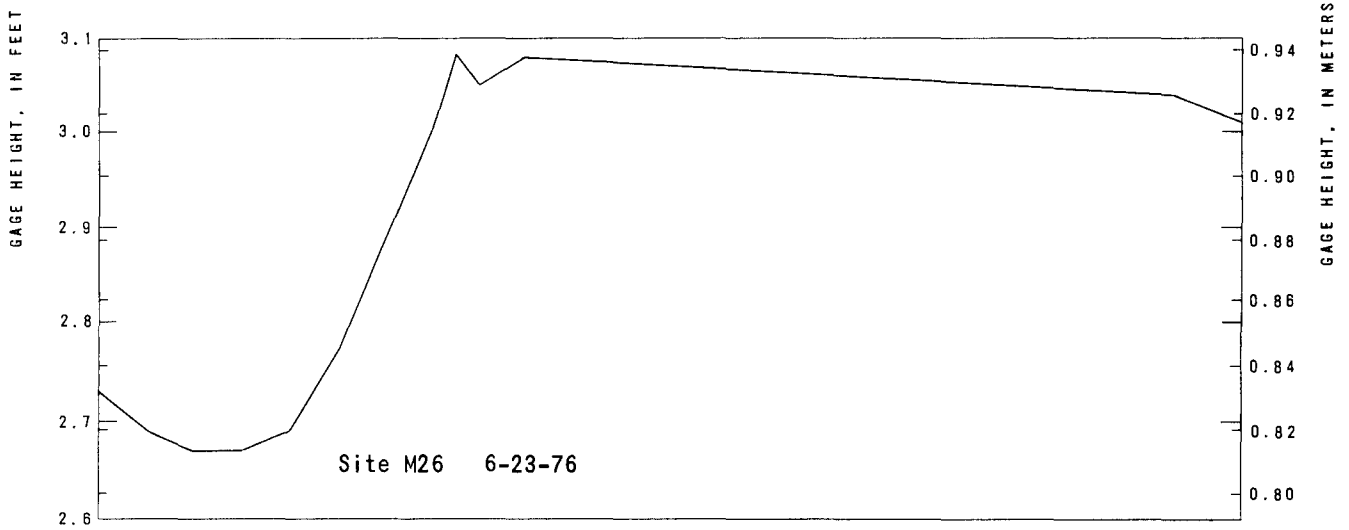
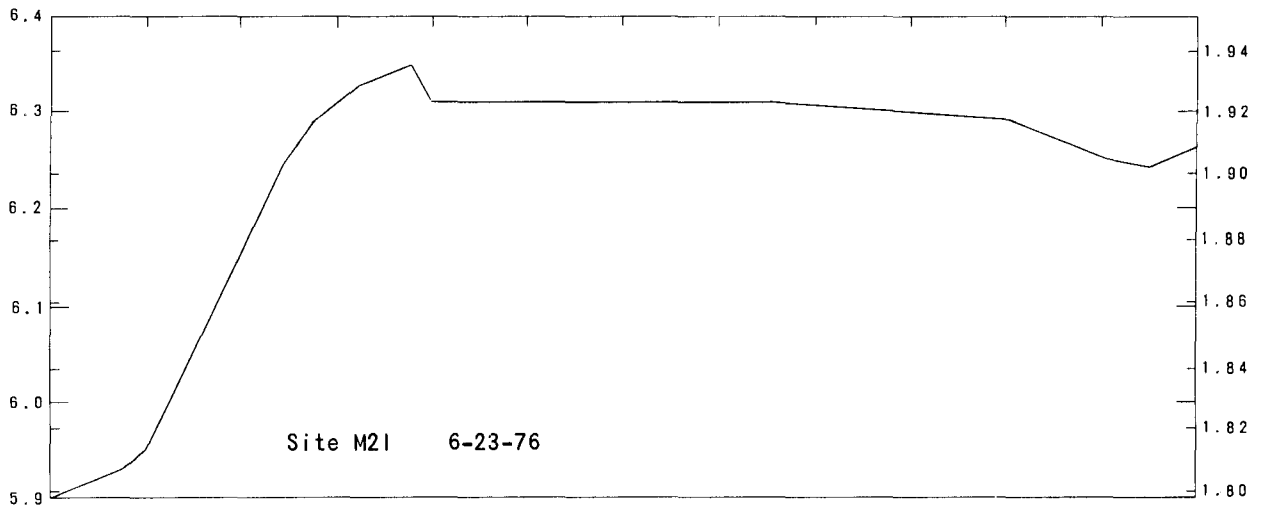


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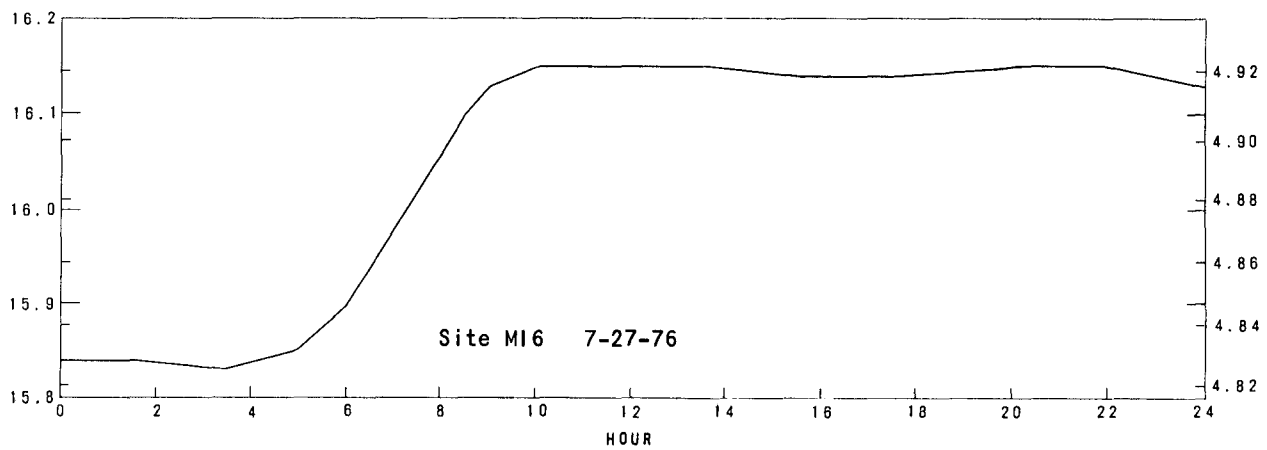
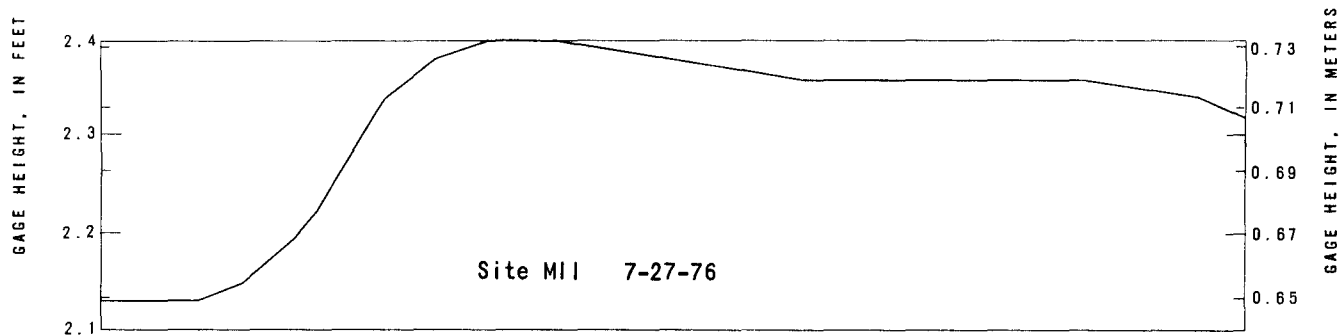
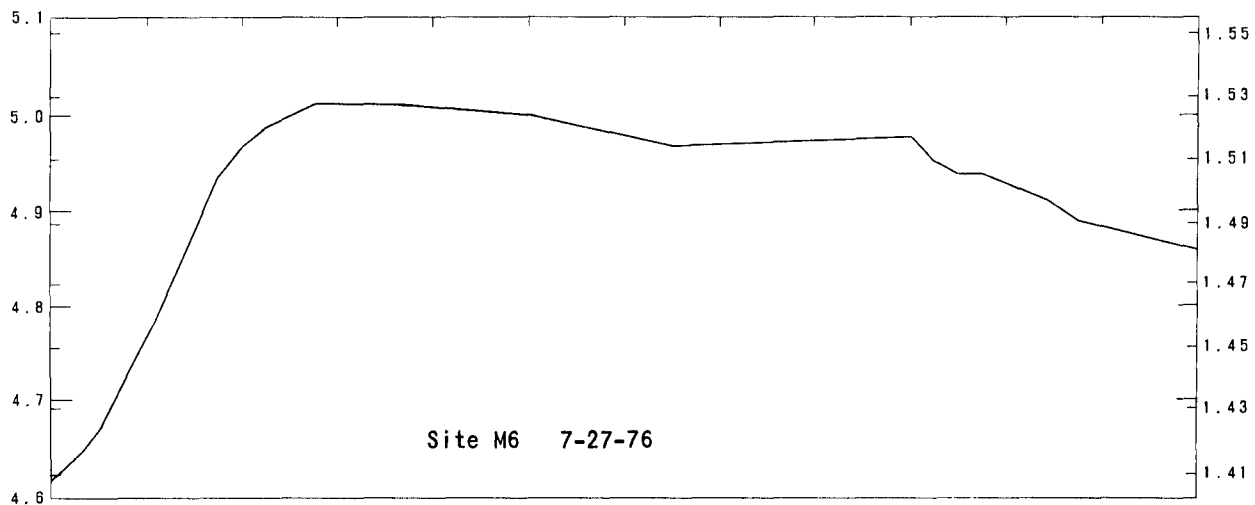


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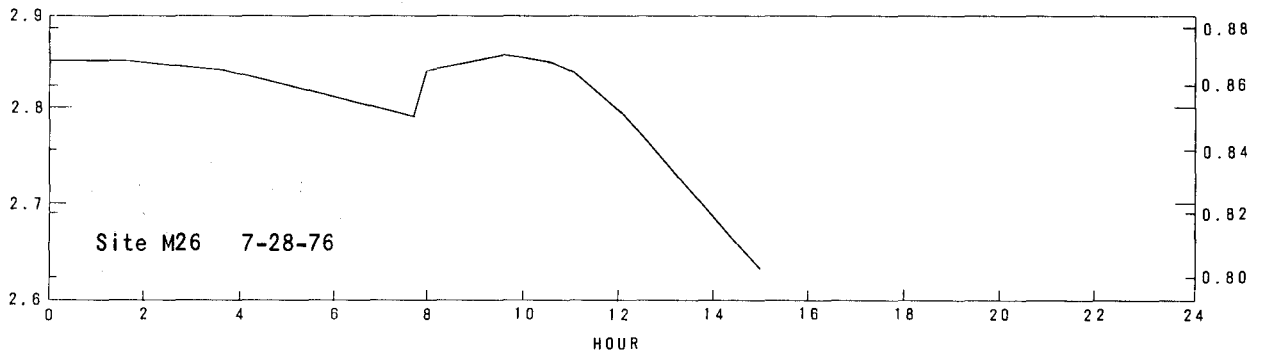
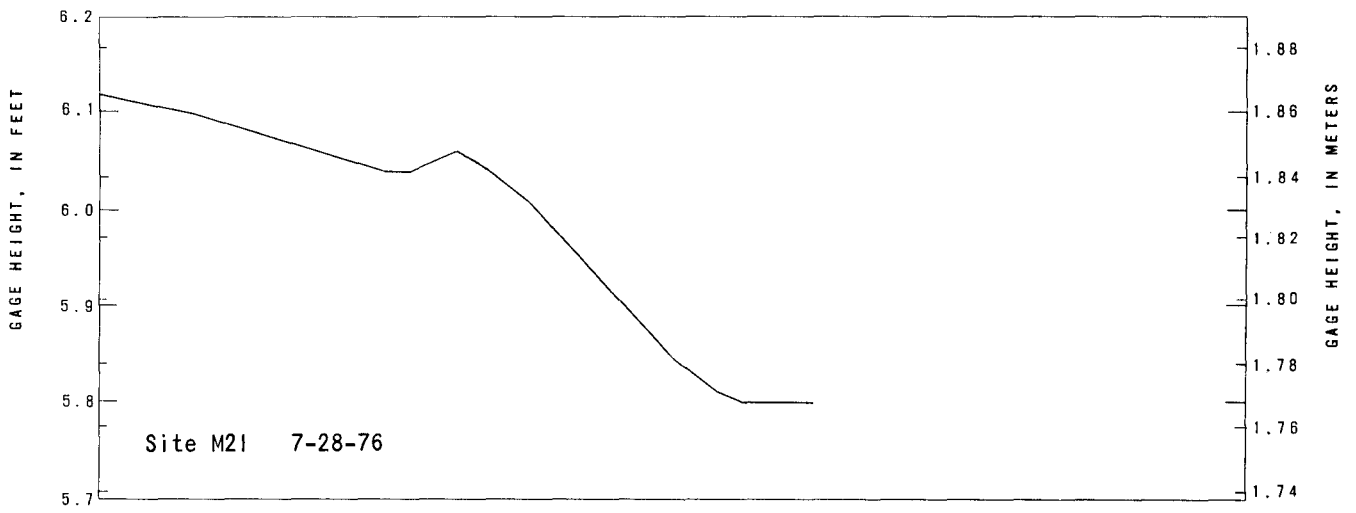
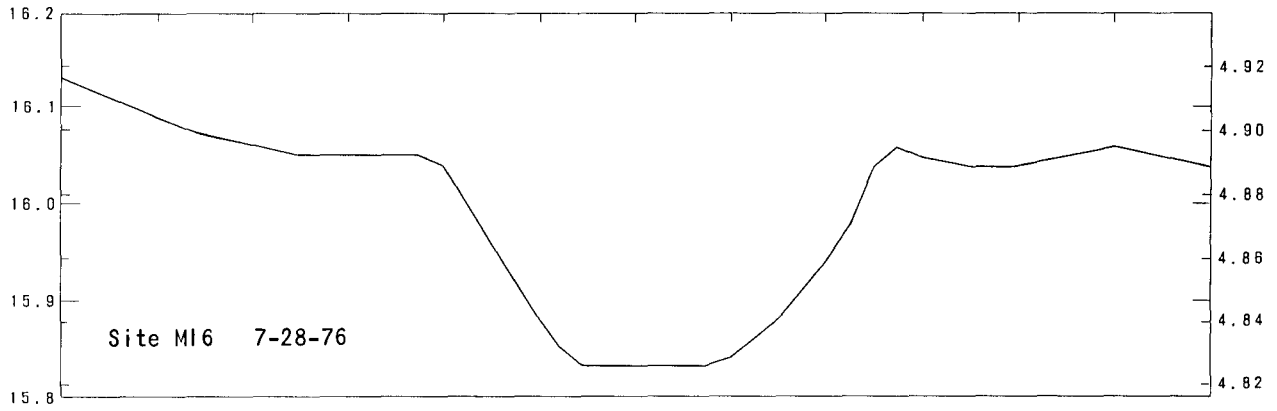
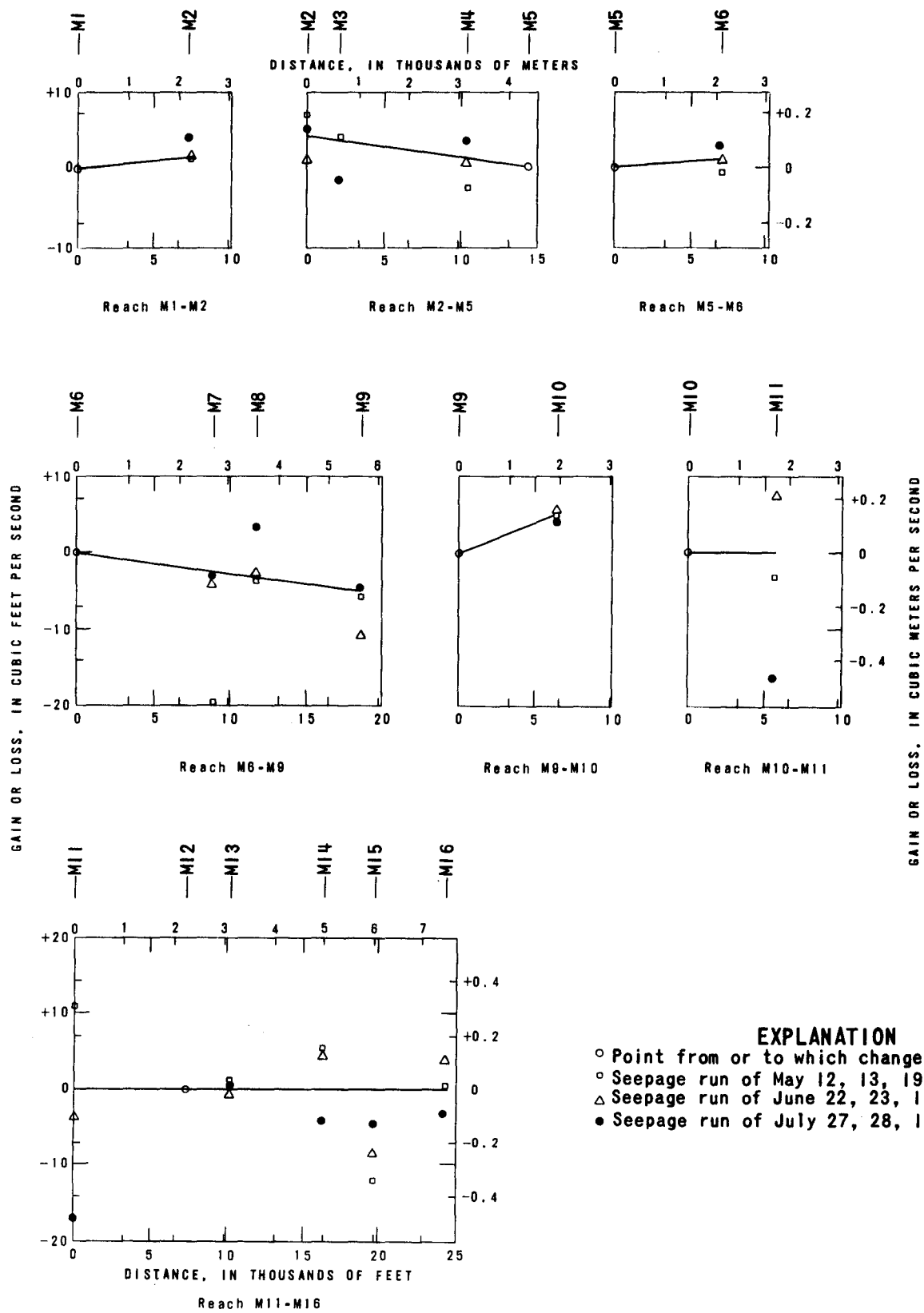


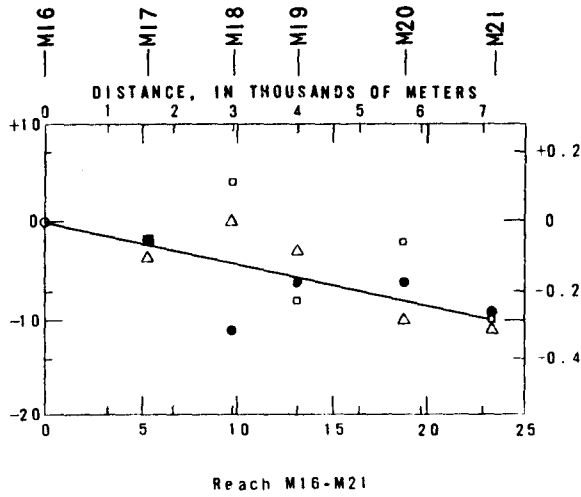
Figure 2.— Continued.



EXPLANATION

- Point from or to which change was computed
- Seepage run of May 12, 13, 1976
- △ Seepage run of June 22, 23, 1976
- Seepage run of July 27, 28, 1976

Figure 3.—Gain or loss for reaches of the canal.



EXPLANATION

- Point from or to which change was computed
- Seepage run of May 12, 13, 1976
- △ Seepage run of June 22, 23, 1976
- Seepage run of July 27, 28, 1976

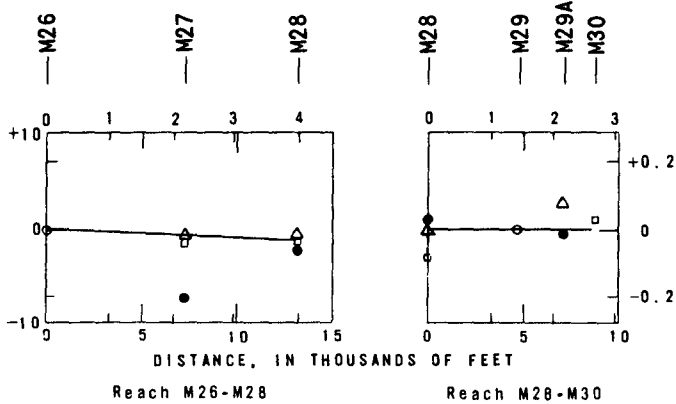
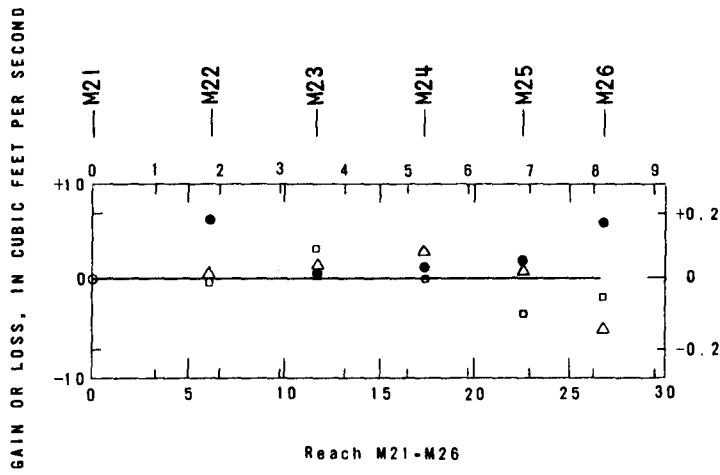


Figure 3.— Continued.

Table 1.--Gain or loss determined from seepage measurements
for reaches of the canal

| Reach | Length (ft) | Graphic average (from fig. 3) | |
|---------|----------------|--|------------------------|
| | | Gain(+) or loss(-) ft ³ /s | (ft ³ /s)mi |
| M1-M2 | 7,250 | +1 | +0.7 |
| M2-M5 | 14,330 | -4 | -1.5 |
| M5-M6 | 7,500 | +1 | +0.7 |
| M6-M9 | 18,580 | -5 | -1.4 |
| M9-M10 | 6,400 | +5 | +4.1 |
| M10-M11 | 5,580 | 0 | 0 |
| M11-M16 | 24,310 | 0 | 0 |
| M16-M21 | 23,450 | -10 | -2.3 |
| M21-M26 | 26,820 | 0 | 0 |
| M26-M28 | 13,470 | -1 | -.4 |
| M28-M30 | 8,720 | 0 | 0 |
| Total | 156,410 | -13 | |

Table 2.--Measurements made on the canal

Site: M, main canal; R, inflow; T, diversion.
 Discharge: e, estimated.

| Site | Date | Time | Discharge (ft ³ /s) | Water temper- ature (°C) | Specific conductance (micromhos per cm at 25°C) |
|------|-------------|------|-----------------------------------|-----------------------------------|--|
| | <u>1976</u> | | | | |
| | May 12 | | | | |
| M1 | | 0940 | 275.1 | 12.0 | 440 |
| M2 | | 1100 | 276.1 | 13.5 | 405 |
| M3 | | 1225 | 273.0 | 14.0 | 405 |
| T4 | | 1335 | 6.33 | - | - |
| T5 | | 1305 | 2.08 | - | - |
| M4 | | 1530 | 262.7 | 15.5 | 420 |
| M5 | | 1650 | 265.2 | 15.5 | 405 |
| M6 | | 1810 | 264.4 | 15.5 | 420 |
| M6 | | 0855 | 265.4 | 9.5 | 390 |
| M7 | | 1010 | 245.9 | 11.5 | 405 |
| M8 | | 1110 | 261.0 | 12.5 | 420 |
| M9 | | 1230 | 257.5 | 12.0 | 410 |
| T16 | | 1320 | 3.16 | - | - |
| T17 | | 1340 | 4.98 | - | - |
| T18 | | 1410 | 10.1 | - | - |
| T19 | | 1435 | 3.21 | - | - |
| T19A | | 1500 | 3.48 | - | - |
| M10 | | 1540 | 235.4 | 14.0 | 420 |
| T20 | | 1620 | 3.71 | - | - |
| T21A | | 1650 | .37 | - | - |
| T22 | | 1710 | 4.86 | - | - |
| T23 | | 1730 | 5.39 | - | - |
| M11 | | 1810 | 219.1 | 15.0 | 410 |
| M11 | | 0900 | 236.5 | 12.0 | 420 |
| T25 | | 0950 | 3.10 | - | - |
| M12 | | 1050 | 220.3 | 12.0 | 435 |
| M13 | | 1240 | 218.1 | 12.5 | 405 |
| M14 | | 1530 | 220.5 | 14.0 | 420 |
| T28 | | 1615 | 4.16 | - | - |
| M15 | | 1730 | 195.0 | 14.5 | 400 |
| T30 | | 1820 | 3.85 | - | - |
| M16 | | 1935 | 207.2 | 14.0 | 420 |
| M16 | May 13 | 0830 | 203.8 | 10.0 | 405 |
| T34 | | 0900 | .7e | - | - |
| T34A | | 0920 | .97 | - | - |
| M17 | | 1010 | 200.9 | 13.0 | 415 |

Table 2.--Measurements made on the canal--Continued

| Site | Date | Time | Discharge (ft ³ /s) | Water temper- ature (°C) | Specific conductance (micromhos per cm at 25°C) |
|------|-------------|------|-----------------------------------|-----------------------------------|--|
| | <u>1976</u> | | | | |
| | May 13 | | | | |
| T38 | | 1050 | 0.68 | - | - |
| T40 | | 1110 | 1.37 | - | - |
| T41 | | 1130 | 4.06 | - | - |
| M18 | | 1215 | 203.5 | 13.0 | 410 |
| T45 | | 1250 | 5.62 | - | - |
| T46 | | 1320 | 3.99 | - | - |
| M19 | | 1350 | 179.7 | 15.0 | 400 |
| M20 | | 1450 | 185.7 | 14.0 | 395 |
| T50 | | 1540 | 3.12 | - | - |
| T52 | | 1600 | 2.08 | - | - |
| T53 | | 1620 | 4.61 | - | - |
| T(D) | | 1645 | 4.87 | - | - |
| M21 | | 1750 | 163.8 | 15.0 | 400 |
| M21 | | 0900 | 157.3 | 14.0 | 440 |
| T2 | | 1000 | 4.28 | - | - |
| M22 | | 1050 | 151.5 | 14.5 | 410 |
| T3 | | 1130 | 4.71 | - | - |
| T4 | | 1150 | 3.94 | - | - |
| M23 | | 1230 | 147.6 | 16.0 | 410 |
| T6 | | 1330 | 2.62 | - | - |
| T7 | | 1340 | .01e | - | - |
| T8 | | 1400 | 3.02 | - | - |
| M24 | | 1440 | 138.7 | 16.5 | 420 |
| T9A | | 1520 | 3.20 | - | - |
| T10 | | 1600 | 3.62 | - | - |
| T10A | | 1630 | 4.24 | - | - |
| T11 | | 1655 | 4.24 | - | - |
| M25 | | 1730 | 118.3 | 18.0 | 415 |
| M26 | | 1840 | 120.2 | 18.0 | 410 |
| M26 | | 0915 | 135.7 | 12.0 | 405 |
| T15 | | 1000 | 9.01 | - | - |
| T18 | | 1030 | 4.27 | - | - |
| M27 | | 1135 | 111.0 | 12.0 | 410 |
| T21 | | 1205 | .01e | - | - |
| M28 | | 1250 | 109.1 | 12.5 | 415 |
| M29 | | 1430 | 110.1 | 15.0 | 410 |
| T(I) | | 1515 | 1.46 | - | - |
| M30 | | 1620 | 111.6 | 16.0 | 420 |
| M1 | June 22 | 0910 | 178.7 | 17.0 | 410 |

Table 2.--Measurements made on the canal--Continued

| Site | Date | Time | Discharge (ft ³ /s) | Water temper- ature (°C) | Specific conductance (micromhos per cm at 25°C) |
|------|-------------|------|-----------------------------------|-----------------------------------|--|
| | <u>1976</u> | | | | |
| M2 | June 22 | 1015 | 181.5 | 17.5 | 440 |
| M3 | | 1120 | 151.0 | 18.5 | 415 |
| T4 | | 1200 | .04 | - | - |
| M4 | | 1310 | 137.4 | 18.5 | 410 |
| M5 | | 1430 | 137.3 | 18.5 | 410 |
| T10 | | 1520 | 3.81 | - | - |
| M6 | | 1615 | 134.5 | 19.0 | 400 |
| M6 | | 0850 | 179.6 | 15.5 | 450 |
| M7 | | 0945 | 176.2 | 16.0 | 455 |
| M8 | | 1040 | 178.1 | 16.5 | 450 |
| T13 | | 1110 | .83 | 16.0 | - |
| T13A | | 1120 | .88 | - | - |
| T14 | | 1140 | 1.12 | 17.0 | - |
| M9 | | 1210 | 166.5 | 17.5 | 450 |
| T15 | | 1240 | 1.29 | - | - |
| T19A | | 1300 | 2.26 | 18.0 | - |
| M10 | | 1340 | 159.4 | 17.0 | 450 |
| T20 | | 1415 | .01 | - | - |
| T21A | | 1415 | .01 | - | - |
| T22 | | 1425 | 3.56 | 17.0 | - |
| M11 | | 1530 | 139.9 | 19.0 | 450 |
| M11 | | 0845 | 165.7 | 16.0 | 450 |
| M12 | | 1030 | 167.4 | 17.5 | 455 |
| M13 | | 1200 | 163.6 | 18.0 | 425 |
| M14 | | 1330 | 169.0 | 19.0 | 455 |
| M15 | | 1500 | 154.1 | 18.0 | 450 |
| T30 | | 1550 | 3.12 | - | - |
| M16 | | 1720 | 141.4 | 18.0 | 455 |
| M16 | June 23 | 0900 | 156.4 | 14.5 | 455 |
| T34A | | 0940 | .74 | - | - |
| M17 | | 1015 | 152.1 | 14.5 | 455 |
| T41 | | 1100 | 3.43 | - | - |
| T43 | | 1110 | .11 | - | - |
| M18 | | 1140 | 152.3 | 15.0 | 455 |
| T45 | | 1210 | 4.28 | - | - |
| M19 | | 1240 | 144.9 | 16.0 | 460 |
| T48 | | 1315 | 3.29 | - | - |
| M20 | | 1400 | 134.4 | 18.0 | 450 |
| T51 | | 1445 | 4.34 | - | - |

Table 2.--Measurements made on the canal--Continued

| Site | Date | Time | Discharge (ft ³ /s) | Water temper- ature (°C) | Specific conductance (micromhos per cm at 25°C) |
|------|-------------|------|-----------------------------------|-----------------------------------|--|
| | <u>1976</u> | | | | |
| T(D) | June 23 | 1510 | 1.97 | - | - |
| M21 | | 1600 | 126.4 | 18.0 | 460 |
| M21 | | 0900 | 128.4 | 17.0 | 440 |
| T2 | | 0940 | 2.74 | - | - |
| M22 | | 1010 | 126.0 | 16.5 | 445 |
| T4 | | 1040 | .06 | - | - |
| M23 | | 1110 | 126.6 | 17.0 | 430 |
| M24 | | 1220 | 124.5 | 18.0 | 465 |
| T10A | | 1300 | 4.10 | - | - |
| M25 | | 1340 | 119.7 | 18.5 | 455 |
| T(E) | | 1420 | 2.65 | - | - |
| M26 | | 1500 | 113.8 | 19.0 | 415 |
| M26 | | 0900 | 125.3 | 16.0 | 425 |
| T19 | | 1000 | 2.32 | - | - |
| M27 | | 1045 | 122.2 | 17.0 | 435 |
| T20 | | 1120 | 5.92 | - | - |
| M28 | | 1215 | 116.2 | 17.0 | 430 |
| T24 | | 1255 | 2.98 | - | - |
| M29 | | 1345 | 112.6 | 18.0 | 435 |
| M29A | | 1550 | 114.3 | 19.0 | 420 |
| T(I) | | 1440 | 1.60 | - | - |
| M1 | July 27 | 0810 | 139.7 | 20.5 | 495 |
| T1 | | 0825 | .01 | - | - |
| M2 | | 0940 | 143.5 | 21.0 | 495 |
| M3 | | 1055 | 135.2 | 21.0 | 485 |
| M4 | | 1210 | 140.6 | 22.0 | 490 |
| M5 | | 1345 | 136.9 | 23.0 | 490 |
| T10 | | 1140 | .10 | - | - |
| M6 | | 1515 | 142.5 | 28.5 | 490 |
| M6 | | 0830 | 140.3 | 19.0 | 495 |
| M7 | | 0920 | 137.3 | 19.0 | 500 |
| M8 | | 1020 | 143.1 | 20.0 | 500 |
| T13 | | 1050 | .62 | - | - |
| T13A | | 1110 | .47 | - | - |
| M9 | | 1140 | 133.9 | 20.0 | 500 |
| T17 | | 1210 | 2.84 | - | - |
| M10 | | 1300 | 134.9 | 21.0 | 505 |
| M11 | | 1435 | 117.3 | 22.0 | 495 |
| M11 | | 0900 | 124.9 | 19.0 | 485 |

Table 2.--Measurements made on the canal--Continued

| Site | Date | Time | Discharge (ft ³ /s) | Water temper- ature (°C) | Specific conductance (micromhos per cm at 25°C) |
|------|-------------|------|-----------------------------------|-----------------------------------|--|
| | <u>1976</u> | | | | |
| M12 | July 27 | 1030 | 142.0 | 19.0 | 495 |
| M13 | | 1215 | 140.6 | 20.0 | 495 |
| M14 | | 1400 | 134.9 | 20.0 | 495 |
| M15 | | 1510 | 134.5 | 19.0 | 485 |
| T31 | | 1550 | 7.00 | - | - |
| T33 | | 1755 | .25 | - | - |
| M16 | | 1720 | 131.6 | 21.0 | 500 |
| M16 | July 28 | 0830 | 109.0 | 19.0 | 490 |
| M17 | | 0930 | 102.3 | 19.5 | 490 |
| T41 | | 1010 | 1.65 | - | - |
| M18 | | 1040 | 86.2 | 21.0 | 485 |
| M19 | | 1135 | 89.2 | 20.0 | 490 |
| M20 | | 1240 | 88.2 | 21.0 | 490 |
| T(D) | | 1310 | .06 | - | - |
| M21 | | 1350 | 85.0 | 21.5 | 495 |
| M21 | | 0740 | 104.0 | 20.0 | 470 |
| M22 | | 0900 | 108.6 | 23.0 | 490 |
| M23 | | 1000 | 99.8 | 23.0 | 485 |
| M24 | | 1100 | 98.5 | 23.5 | 475 |
| M25 | | 1215 | 95.1 | 24.0 | 480 |
| M26 | | 1320 | 97.3 | 25.0 | 485 |
| M26 | | 0845 | 115.8 | 20.0 | 485 |
| M27 | | 1000 | 108.3 | 20.0 | 490 |
| T20 | | 1035 | 3.42 | - | - |
| M28 | | 1130 | 109.4 | 21.0 | 485 |
| M29 | | 1230 | 107.1 | 21.0 | 490 |
| T28 | | 1310 | 3.27 | - | - |
| M29A | | 1345 | 99.2 | 22.0 | 490 |
| M1 | Sept. 14 | 0900 | 40.6 | 16.0 | 505 |
| M2 | | 0955 | 39.5 | 16.5 | 500 |
| M3 | | 1040 | 39.8 | 17.0 | 500 |
| T3 | | 1100 | .08 | - | - |
| M4 | | 1125 | 37.8 | 18.0 | 500 |
| M5 | | 1210 | 36.8 | 19.0 | 500 |
| M6 | | 1255 | 37.9 | 19.5 | 515 |
| M6 | | 0840 | 35.5 | 14.0 | 500 |
| M7 | | 0940 | 34.9 | 14.0 | 505 |
| M8 | | 1025 | 34.8 | 14.5 | 500 |
| M9 | | 1125 | 34.8 | 16.0 | 500 |

Table 2.--Measurements made on the canal--Continued

| Site | Date | Time | Discharge (ft ³ /s) | Water temper- ature (°C) | Specific conductance (micromhos per cm at 25°C) |
|------|-------------|------|-----------------------------------|-----------------------------------|--|
| | <u>1976</u> | | | | |
| M10 | Sept. 14 | 1230 | 35.2 | 16.0 | 485 |
| M11 | | 1340 | 35.0 | 17.0 | 475 |
| M11 | | 0920 | 38.5 | 15.0 | 490 |
| M12 | | 1040 | 36.6 | 15.0 | 495 |
| M13 | | 1150 | 38.3 | 15.0 | 490 |
| M14 | | 1330 | 36.1 | 16.0 | 495 |
| M15 | Sept. 15 | 1430 | 38.3 | 17.0 | 485 |
| M16 | | 1545 | 36.1 | 18.0 | 490 |
| M16 | | 0830 | 20.3 | 15.0 | 510 |
| R1 | | 0910 | .70 | - | - |
| M17 | | 0940 | 18.7 | 16.0 | 510 |
| M18 | | 1020 | 19.1 | 16.0 | 515 |
| M19 | 1105 | 18.6 | 16.0 | 530 | |
| M20 | 1150 | 18.4 | 16.0 | 535 | |
| T(D) | 1220 | .85 | - | - | |
| M21 | 1300 | 15.1 | 16.0 | 555 | |
| M21 | 0850 | 24.4 | 18.0 | - | |
| M22 | 0935 | 24.1 | 18.5 | 515 | |
| M23 | 1020 | 24.1 | 18.5 | 500 | |
| M24 | 1110 | 23.3 | 18.5 | 500 | |
| M25 | 1200 | 23.4 | 19.0 | 500 | |
| M26 | 1250 | 21.6 | 21.0 | 500 | |
| M26 | 0855 | 26.6 | 15.0 | 490 | |
| M27 | 1000 | 25.5 | 16.0 | 490 | |
| M28 | 1100 | 26.1 | 16.0 | 490 | |
| M29 | 1200 | 26.2 | 16.0 | 485 | |
| T(I) | 1230 | 1.70 | - | - | |
| M30 | 1315 | 24.3 | 17.0 | 495 | |

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